



Failure of Concrete Foundations On Non-Expansive Fill Over Expansive Soils

Case Study
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In a nutshell:

Within 90 days of completion of 20 single-story 2,000 sf homes, the foundations began to 'fail,' causing cracks up walls, doors that wouldn't open, and windows that wouldn't close

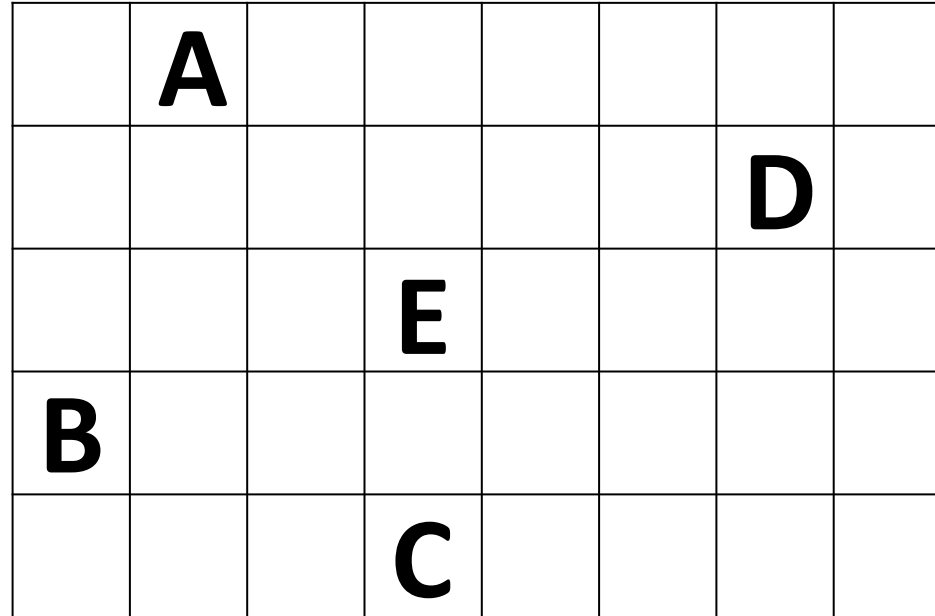
Conclusions:

Use of non-expansive fill did not stop and/or reduce underlying expansive soil uplift forces on the installed slabs



The situation

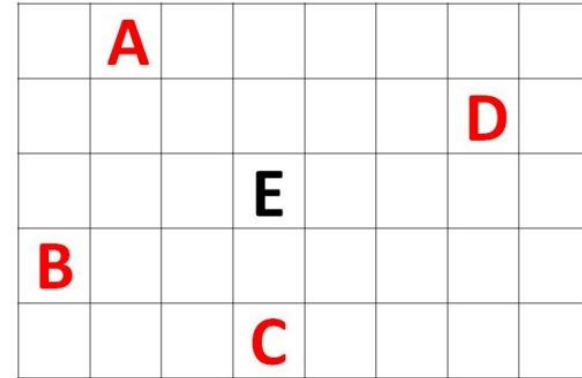
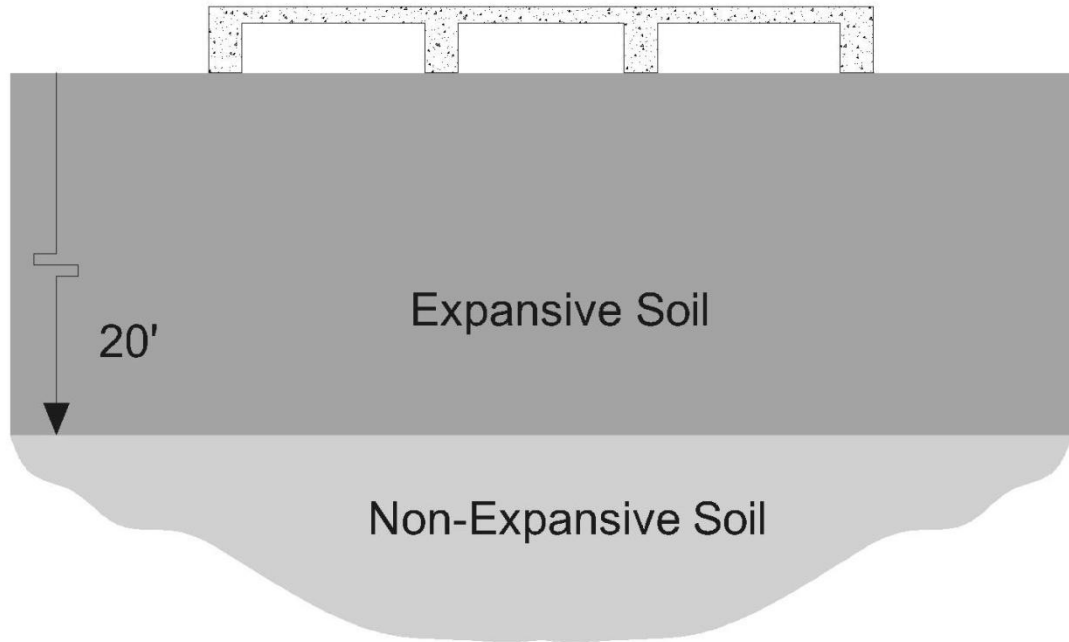
Five neighborhoods in close proximity were all built on expansive soils



- Similar expansive soil conditions
- Same void-space product used on all



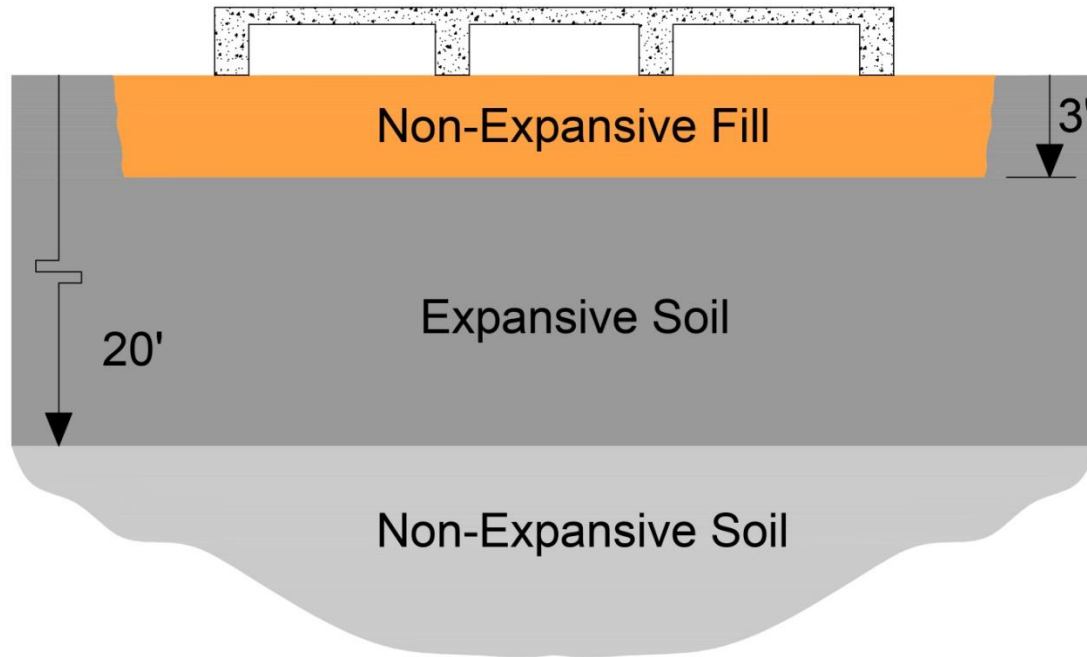
Building sites A, B, C, D



- Foundations built on expansive soil
- No non-expansive fill
- No reported foundation 'failures'



Building site E



	A				
					D
			E		
B					
			C		

- 3' of expansive soil was removed and replaced with non-expansive fill that was compacted to minimum 90+%
- 20 of these foundations 'failed'



Why?

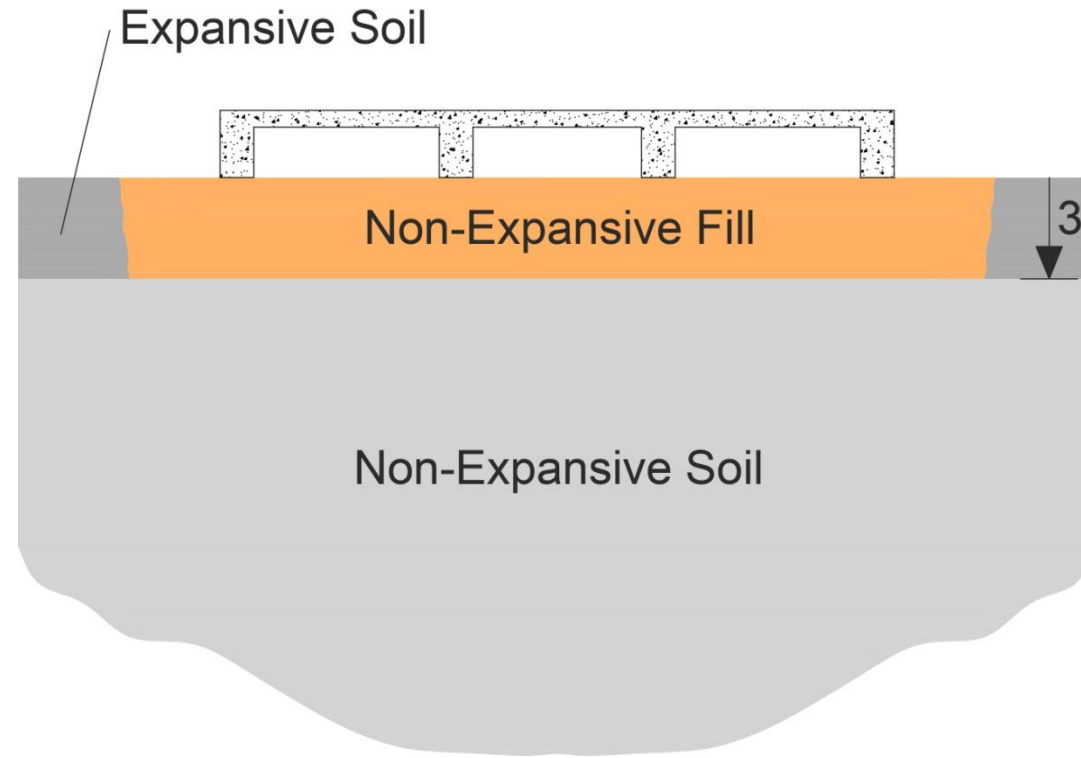


Purpose of using non-expansive fill layer

- Reduce vertical rise of underlying soils
- Reduce amount of materials used in foundation (concrete, rebar, PT cables)
- Reduce building cycletime vs. alternative slabs

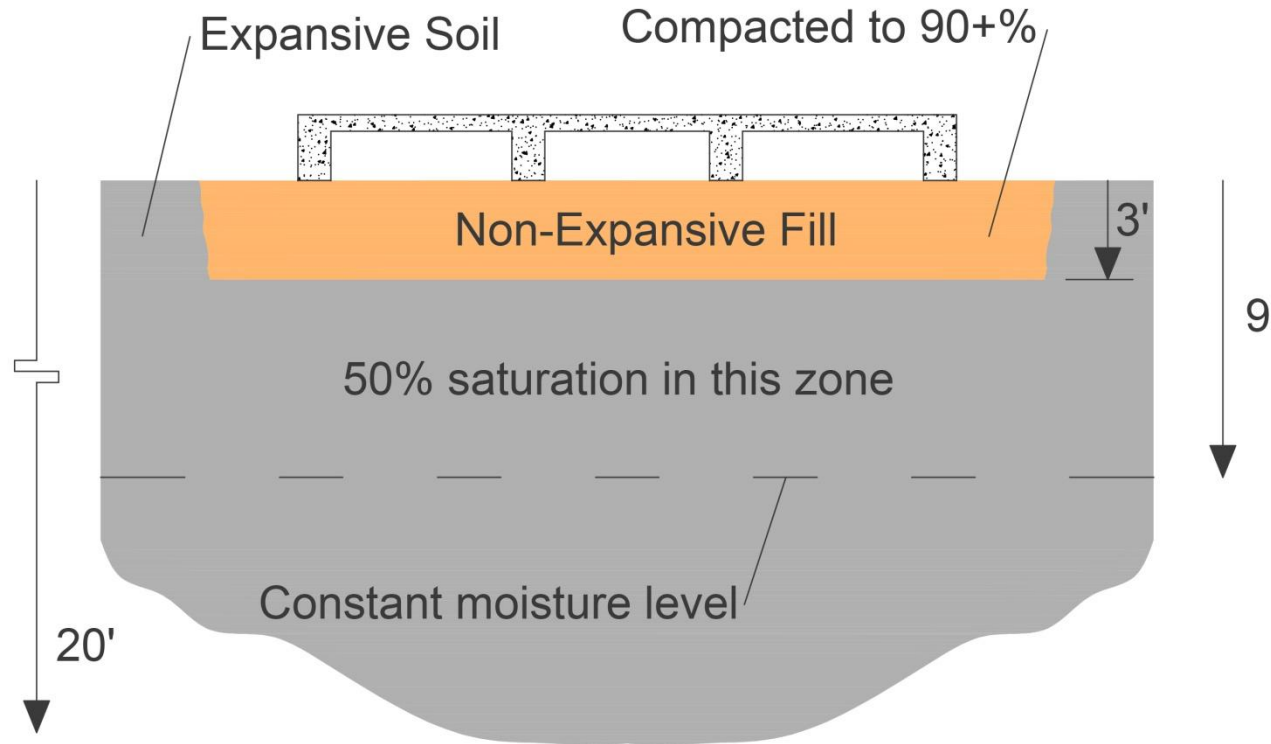


Where replacement of expansive soil with non-expansive fill can work



- Removal of the 3' deep layer of expansive soils completely eliminates expansive soil problems

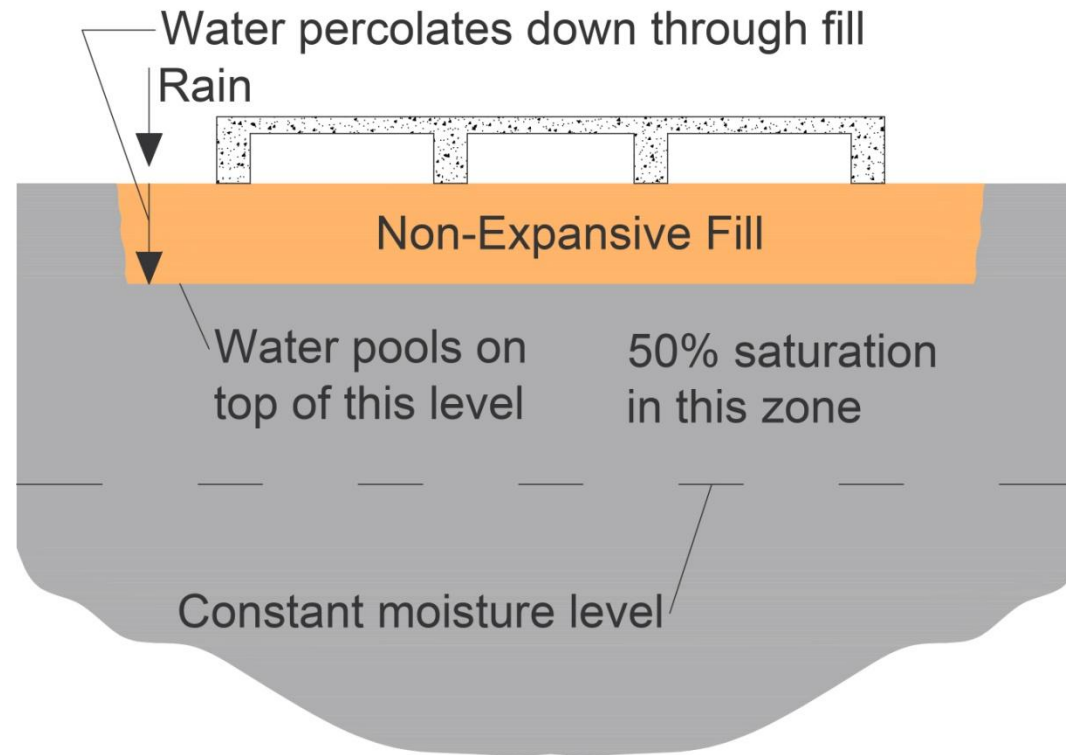
Where replacement of expansive soil with non-expansive fill does not work



- Removing 3' of expansive soil does not significantly reduce the potential expansion of the deep underlying soil



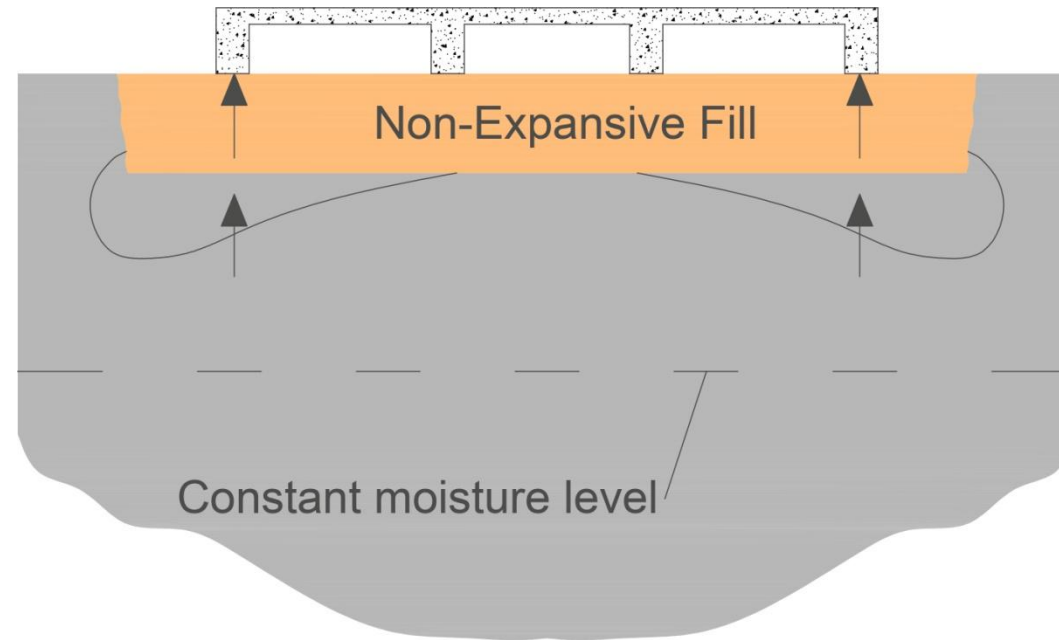
What happens when it rains?



- Non-expansive fill is porous
- It does not prevent water from quickly reaching the underlying expansive soil with 50% saturation

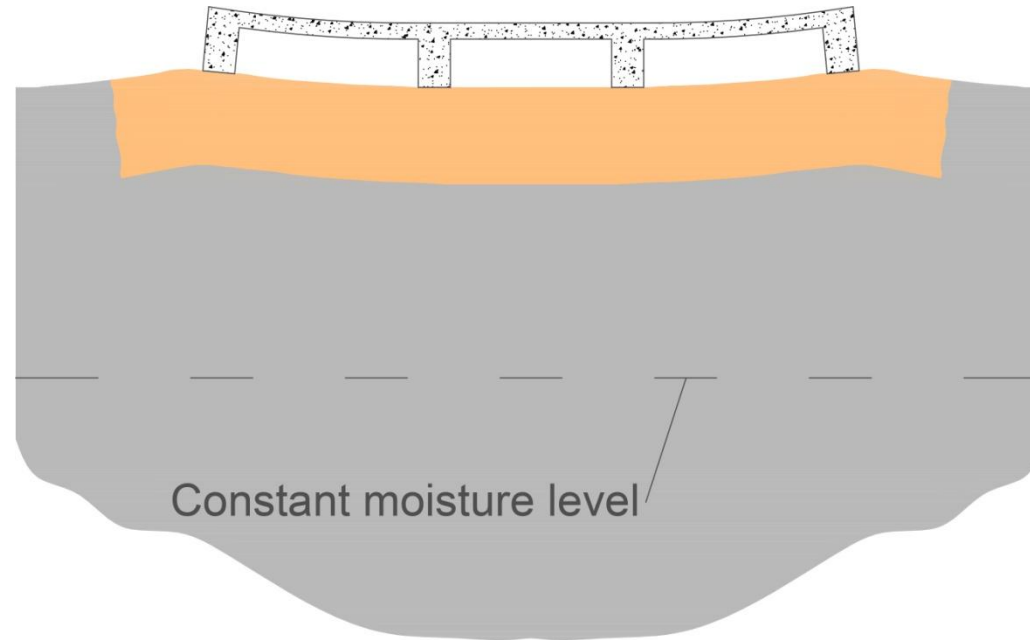


Expansive soil with 50% saturation absorbs water & expands



- Non-expansive fill has a high friction angle
- When compacted, it transmits the uplift force from the underlying expansive soil to the foundation

Exaggerated deformation of foundation and soil uplift



- Foundation perimeter pushed up first
- Water migrates toward the center of foundation



**How is use of
non-expansive fill
supposed to reduce
expansive soil effects?**



PTI Manuals:

- Design of Post-Tensioned Slabs-on-Ground
- “*Standard Requirements for Design and Analysis of Shallow Post-Tensioned Concrete Foundations on Expansive Soils*” allow determination of E_m & Y_m to be made using non-expansive fill layers



**Using PTI method, with
non-expansive fill,
allows a significant
reduction in E_m and Y_m**

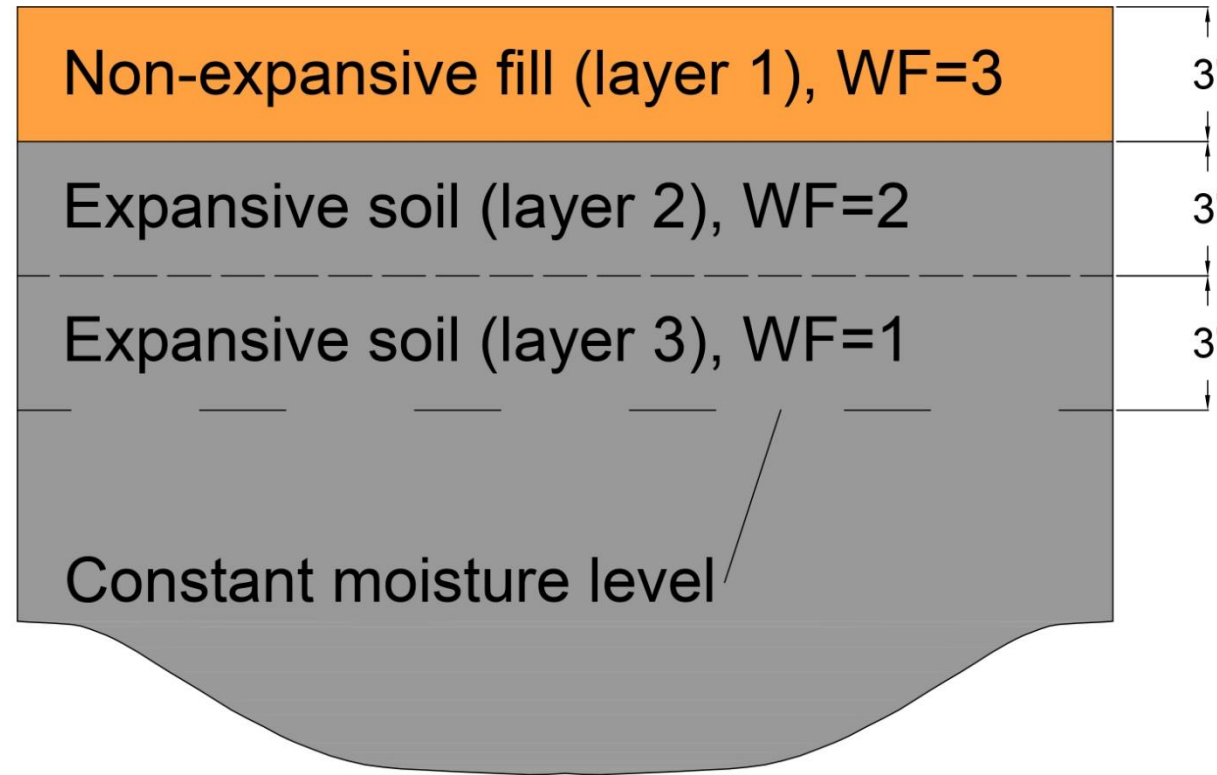


PTI manual allows weighting factors to be used for the layers

- Allows use of multiple layers of different types of soil in determining E_m and Y_m
- Uses 3 layers in equal depth down to 9' total
- Layers have weighting factors of 3, 2, and 1 from top to bottom



PTI weighting methodology



PTI calculation method for E_m and Y_m

- g_h = Change of soil volume for a change in suction corrected for actual % fine clay. Also referred to as Matrix Suction Compression Index (PTI definition)
- PTI states: *“If g_h varies by more than 10%, a computer modeling program is required to accurately calculate Y_m . Non-expansive layers shall be modeled using g_h equal to 0.01.”*



PTI calculation method for E_m and Y_m

- PTI Commentary states: *“This method should only be used if a typical trumpet-shaped final suction profile can be assumed, and g_h does not vary by more than 10% between layers in the soil profile. Otherwise, this method may not be accurate.”*



**What happened to the
foundations in this
case study?**



Comparison of E_m and Y_m calculations

- With non-expansive fill

$$E_m = 9.0' \text{ CL} \quad / \quad 5.2' \text{ EL}$$

$$Y_m = -0.08'' \text{ CL} \quad / \quad 0.66'' \text{ EL}$$

With no fill

$$E_m = 7.8' \text{ CL} \quad / \quad 4.1' \text{ EL}$$

$$Y_m = -0.19'' \text{ CL} \quad / \quad 0.96'' \text{ EL}$$

Remember:

E_m = Edge Moisture Variation Distance

Y_m = Maximum Unrestrained Differential Soil Movement

(These values are used to design the foundation)



Why did foundations on non-expansive fill fail?

- Foundations in four neighborhoods without non-expansive fill did not fail. They were designed for the in-situ expansive soil with higher E_m and Y_m values
- Foundations in one neighborhood used non-expansive fill and 20 'failed.' They were designed with lower E_m and Y_m values



Why did foundations on non-expansive fill fail?

- Non-expansive fill is granular with high friction angle. When compacted to 90+%, it becomes rigid
- Water quickly percolates down through non-expansive fill and pools on top of the in-situ expansive soil that has 50% saturation
- The underlying expansive soil absorbs the water, expands, and pushes up against the compacted non-expansive fill



Why did foundations on non-expansive fill fail?

- Expansive soil has high percentage of fines and a low friction angle
- When wetted, expansive soil becomes plastic in nature and easily flows into void spaces
- The granular fill above is pushed upward, but does not easily flow into void spaces, resulting in higher transmission of uplift force



Conclusions

- When in-situ expansive soil is deep and has low saturation, don't expect reductions in E_m and Y_m from a non-expansive fill layer to work
- Non-expansive fill transmits uplift forces of underlying expansive soil
- Compacted non-expansive fill does not flow into void space
- Expansive soil has a low friction angle and can easily flow into void spaces when wetted, thereby reducing uplift force on the foundation





This case study was presented for developers, engineers, and contractors by the Biax foundation-forming system, a 4th generation, on-grade, waffle slab designed and engineered specifically for expansive, compressible, or rocky soils. A Biax slab delivers superior performance and significant hard-dollar cost savings – especially when compared to in-ground ribbed and/or uniform thickness foundations.

For more information on Biax go to www.biax.com.au or call Tom Richards at 925.683.2739